

FOUNDATION DESIGN SUBJECT TO CHANGE PENDING VIBRATORY STONE COLUMNS DESIGN

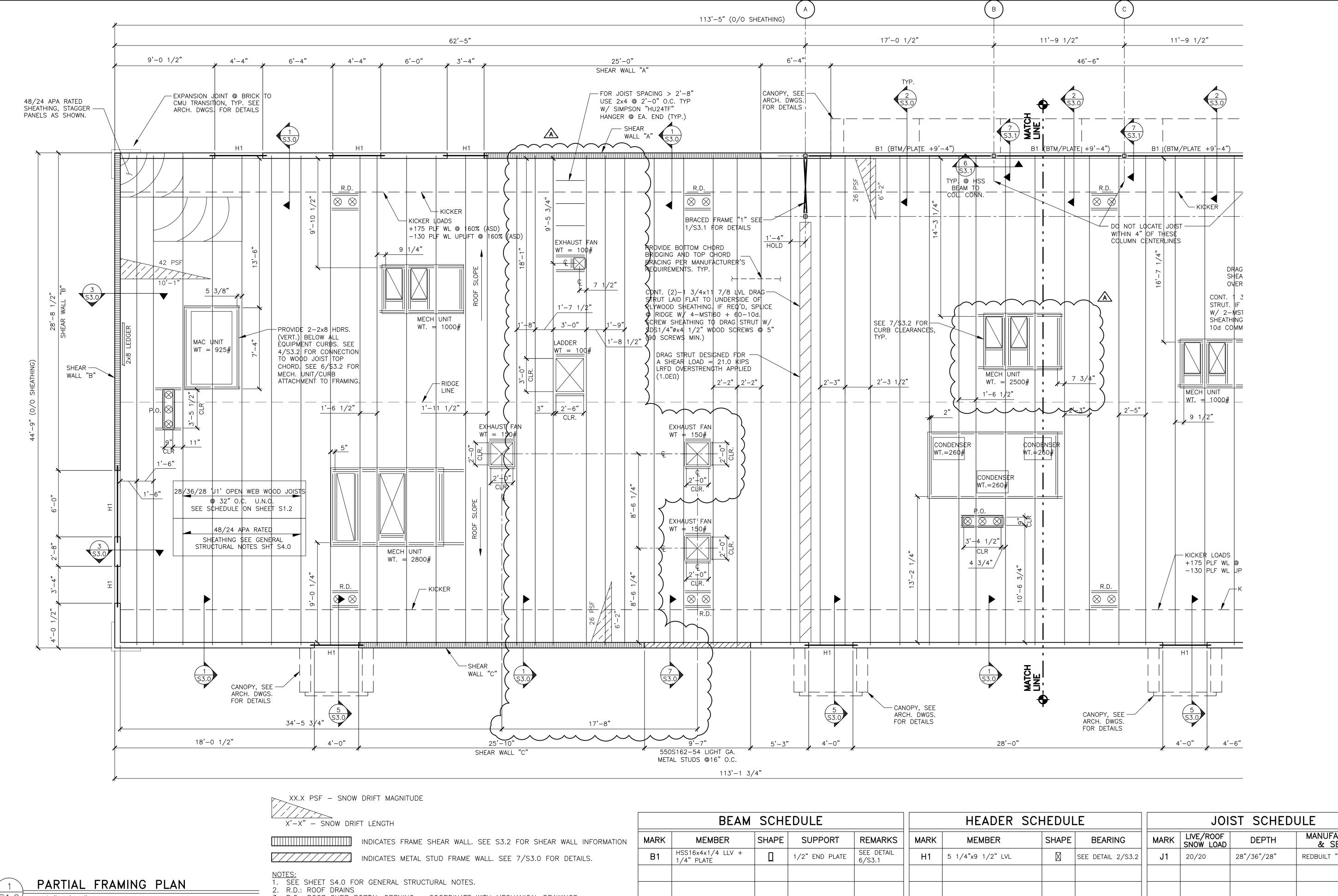
"RPS28Z" FOR NOTCH < 12", W/ 16d NAILS INTO SILL PLATE ENDS. (MAX.

SPACING BETWEEN STUDS = 16" O.C.)

8. WHERE SILL PLATES ARE NOT CONTINUOUS AT SHEAR WALLS, CONTACT ENGINEER OF RECORD FOR RESPONSE.

FOOTING SCHEDULE				PIE	4 SCHEDOLE	COLUMN & BASE PLATE SCHEDULE					
MARK	SIZE	REINFORCING	MARK	SIZE	REINFORCING	REMARKS	MARK	MEMBER	SHAPE	BASE PLATE	REMARKS
F1	4'-0"x4'-0"x1'-0"	5-#5 E.WB.	P1	1'-4"x1'-4"	4-#6 VERT. #4 TIES @ 12" O.C.	SEE DETAIL 8/S2.0	C1	HSS4x4x1/4	+	А	EXTEND TO ROOF
F2	3'-0"x3'-0"x1'-0"	4-#5 E.WB.					C2	HSS4x4x5/16	+	В	BRACED FRAME
F3	6'-0"x15'-0"x3'-0"	14-#7 L.WT&B #6 @ 12" O.C. S.WT&B					С3	HSS4x4x5/16	+	А	EXTEND TO ROOF
F4	5'-0"x11'-0"x3'-0"	11-#7 L.WT&B #6 @ 12" O.C. S.WT&B									
							NOTE: SEE	E DETAIL 3/S3.1 FOR BA	SE PLATE A	AND ANCHO	R ROD DETAILS

FOUNDATION PLAN



1/4"=1'-0"

NOTE TO CONTRACTOR: ANY SPRINKLER LINES GREATER THAN 3" DIAMETER SHALL BE VERIFIED BY THE SPRINKLER CONTRACTOR PRIOR TO TRUSS FABRICATION.

3. P.O.: ROOF CURB PORTAL OPENING - COORDINATE WITH MECHANICAL DRAWINGS.

4. WHEN TOP PLATES ARE NOT CONTINUOUS, USE SIMPSON "RPS22" TIE FOR NOTCH < 5 1/2", USE SIMPSON "RPS28" TIES FOR NOTCH < 12", W/ 16d NAILS INTO TOP PLATE ENDS. (MAX. SPACING BETWEEN STUDS = 16" O.C.)

5. TOP PLATE SPLICE SHALL BE MINIMUM LENGTH OF 48" WITH 2 ROWS (2 1/2" APART) OF 22-16d COMMON NAILS SPACED @ 2" ON CENTER (44 TOTAL).

6. SEE PLAN & 5/S3.2 FOR SNOW DRIFT DIAGRAM.

BEAM SCHEDULE					HEADER SCHEDULE					JOIST SCHEDULE				
MARK	MEMBER	SHAPE	SUPPORT	REMARKS	MARK	MEMBER	SHAPE	BEARING	MARK	LIVE/ROOF SNOW LOAD	DEPTH	MANUFACTURER & SERIES		
B1	HSS16x4x1/4 LLV + 1/4" PLATE		1/2" END PLATE	SEE DETAIL 6/S3.1	H1	5 1/4"x9 1/2" LVL		SEE DETAIL 2/S3.2	J1	20/20	28"/36"/28"	REDBUILT "RED-S"		
												S AND SUPPLIER INFORMATION		

NOTE: SEE DETAIL 4/S3.2 FOR JOIST DETAILS AND SUPPLIER INFORMATION

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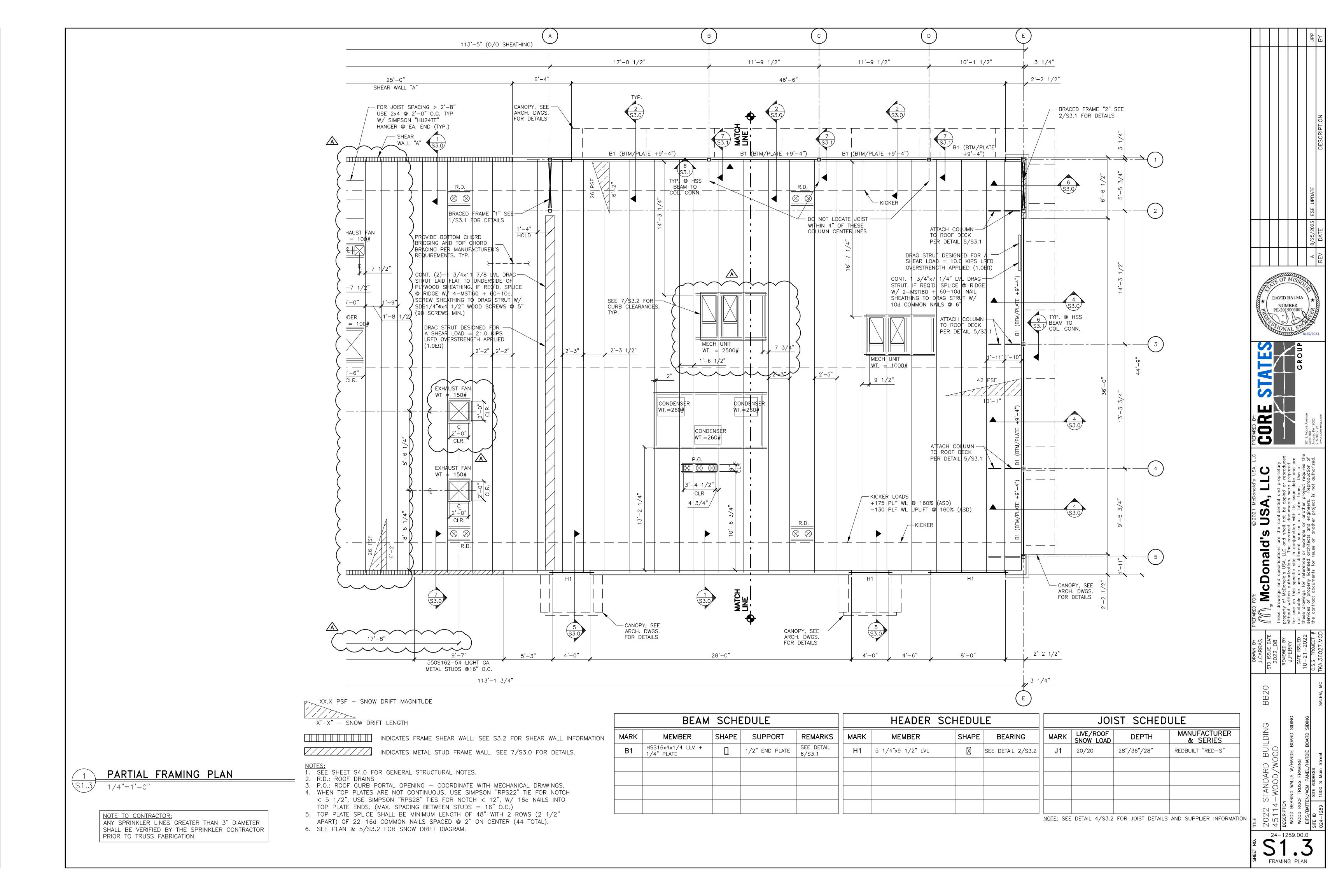
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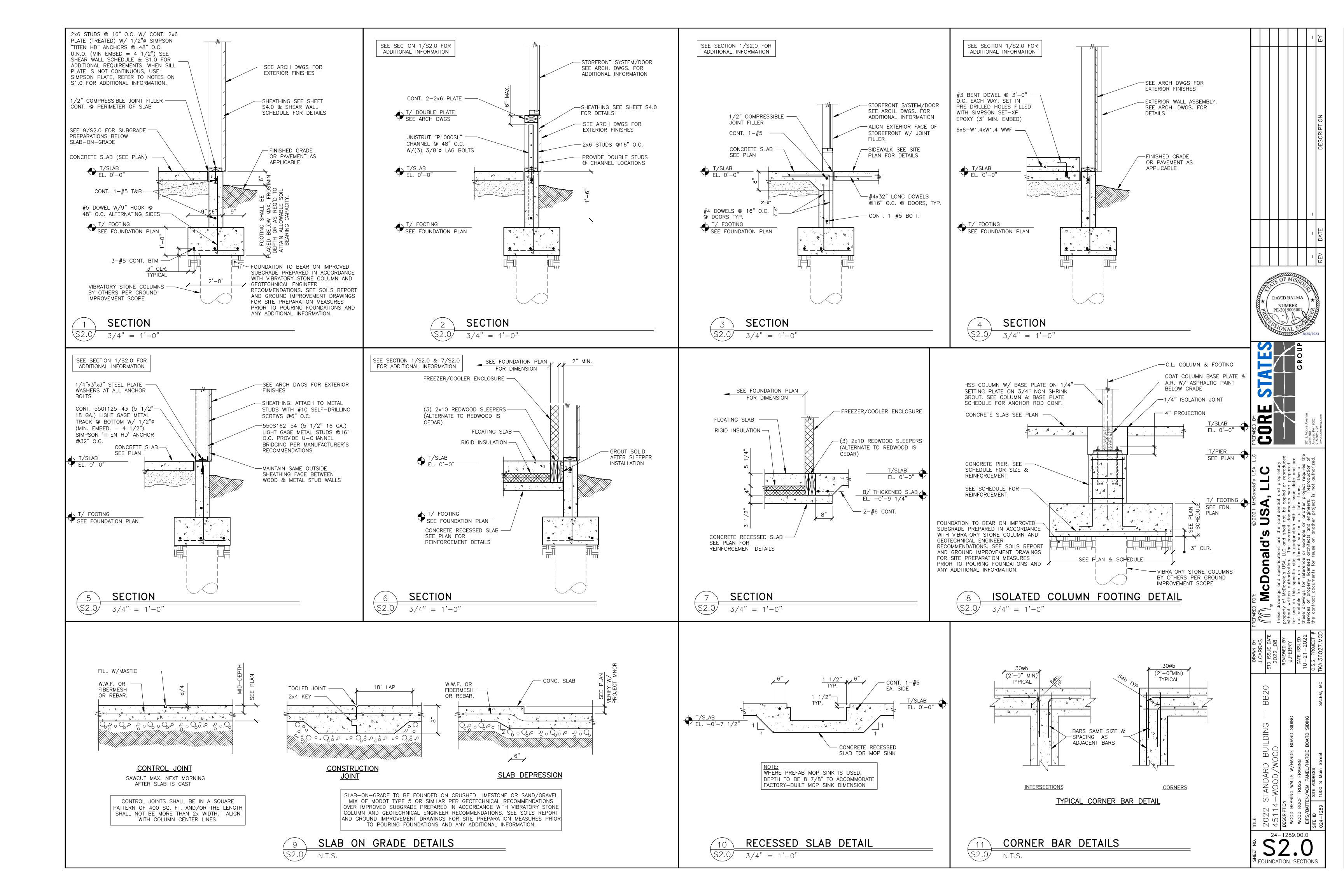
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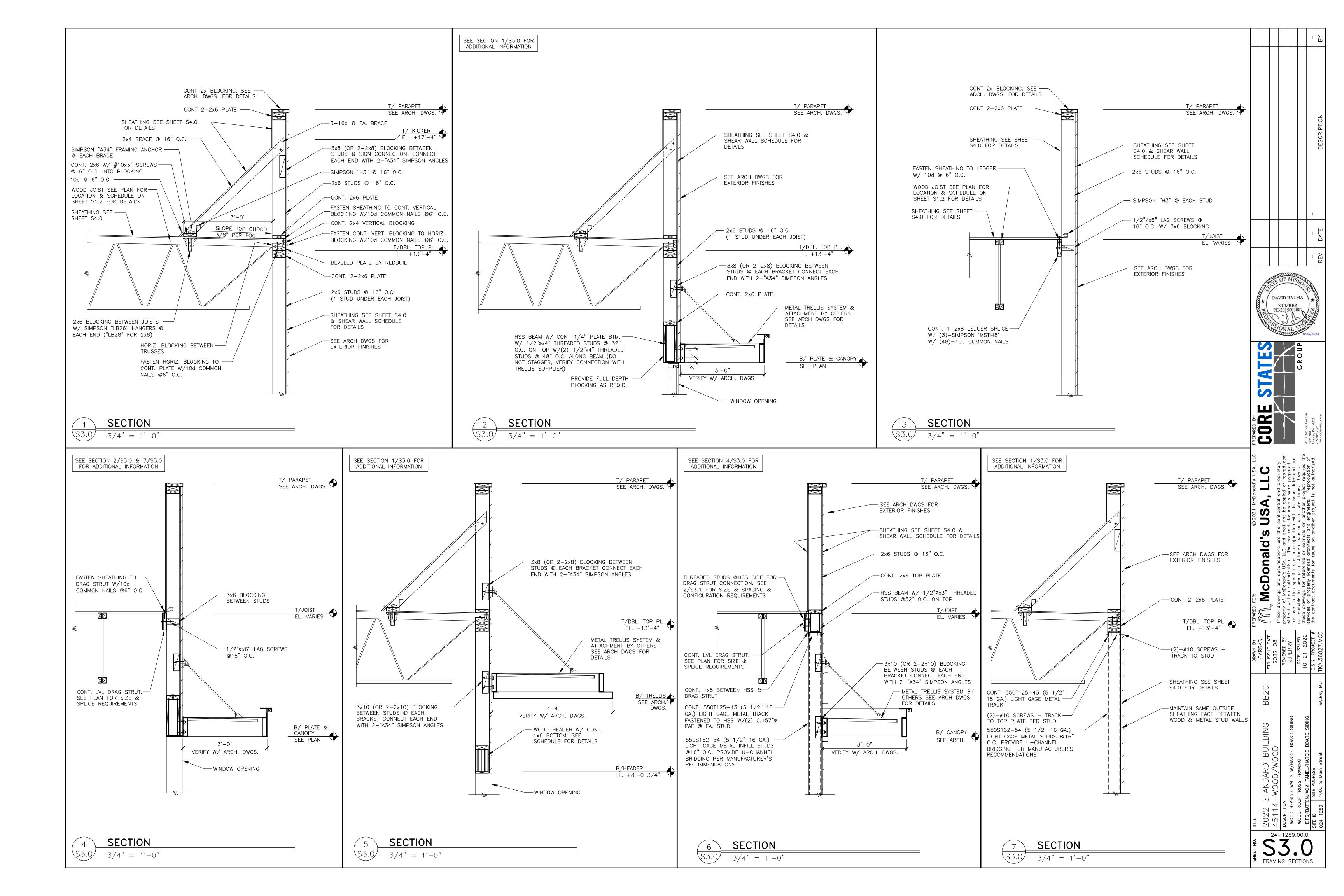
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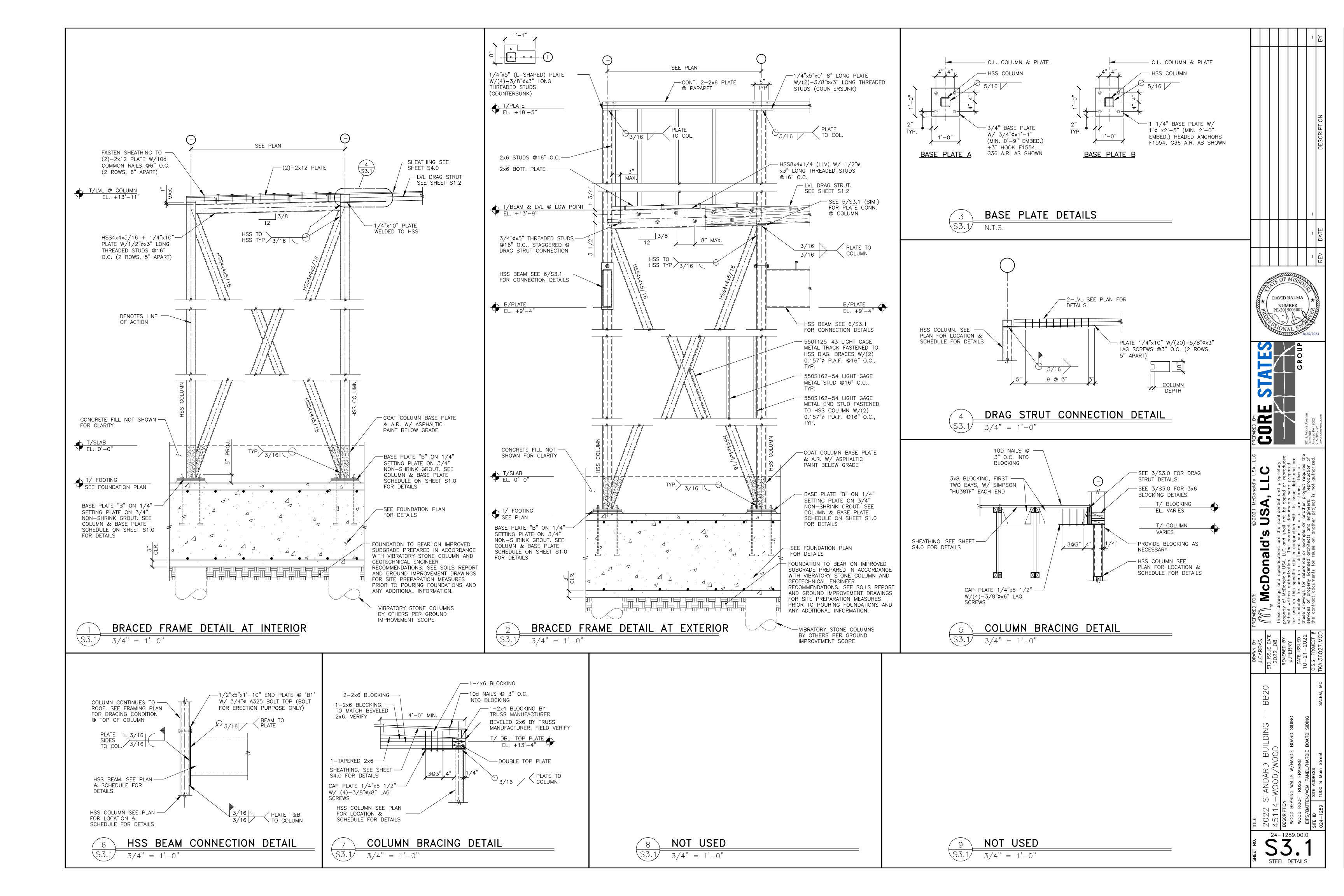
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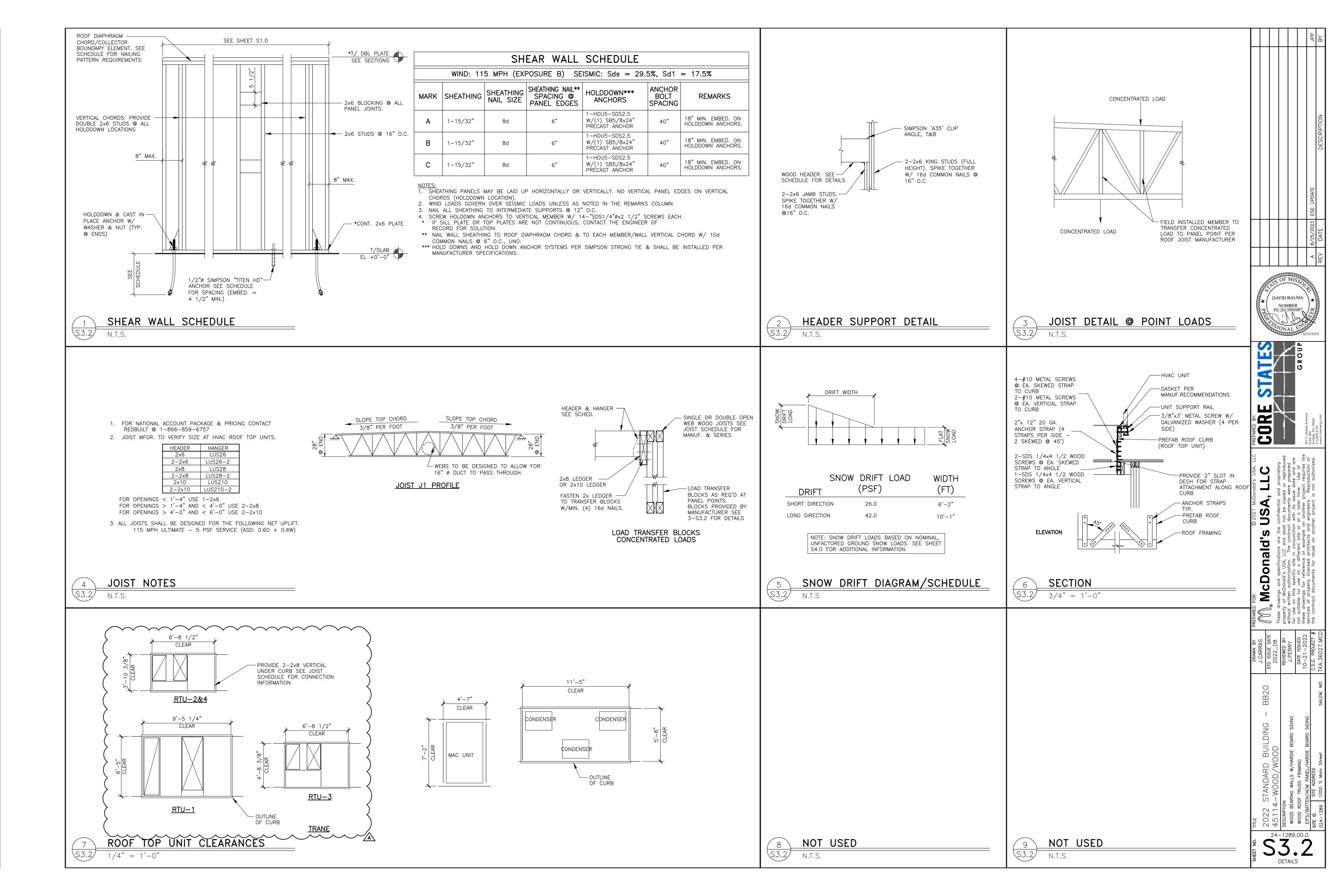
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STRUCTURAL GENERAL NOTES: SAWN LUMBER ALL GRADES OF LUMBER INDICATED ON STRUCTURAL DRAWINGS SHALL BE RATED BY THE . THE STRUCTURAL DESIGN OF THIS BUILDING WAS BASED ON THE DESIGN CRITERIA: A. BUILDING CODE: 2012 INTERNATIONAL BUILDING CODE SOUTHERN PINE INSPECTION BUREAU (SPIB), OR THE WESTERN WOOD PRODUCTS B. FLOOR: ASSOCIATION (WWPA). LUMBER GRADES SHALL BE AS FOLLOWS, WITH A MAXIMUM LIVE LOAD: 100 PSF MOISTURE CONTENT OF 19%: C. ROOF: LIVE LOAD: 20 PSF A. SOUTHERN PINE NO. 1. DEAD LOAD: 20 PSF B. DOUGLAS FIR-LARCH NO. 1. D. SNOW: C. HEM-FIR NORTH NO. 1 GROUND LOAD: 20 PSF FLAT ROOF LOAD: 20 PSF BOLT HEADS AND NUTS BEARING ON WOOD SHALL BE PROVIDED WITH STANDARD CUT SNOW EXPOSURE FACTOR, CE: 1.0 WASHERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE IMPORTANCE FACTOR, I: 1.0 THERMAL COEFFICIENT, CT: 1.0 E. WIND: MINIMUM NAILED CONNECTIONS FOR WOOD FRAMING MEMBERS SHALL BE IN ACCORDANCE ULTIMATE WIND SPEED: 115 MPH (3-SECOND GUST) WITH THE LOCAL BUILDING CODE OR TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING RISK CATEGORY: II CODE IF NO OTHER CRITERIA IS GIVEN. WIND EXPOSURE: B PRESSURES PER ASCE 7-10 CONNECTORS SHOWN ON THE DETAILS ARE MANUFACTURED BY SIMPSON. WRITTEN F. SEISMIC: APPROVAL BY ENGINEER REQUIRED FOR SUBSTITUTIONS.

ALL SHEATHING SHALL CONFORM TO AMERICAN PLYWOOD ASSOCIATION (APA) DESIGN

WALL SHEATHING SHALL BE 15/32" (1/2" NOMINAL) APA RATED SHEATHING, EXPOSURE 1

COMMON NAILS @ 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATED SUPPORTS,

ROOF SHEATHING SHALL BE 23/32" (3/4" NOMINAL) APA RATED SHEATHING, EXPOSURE 1,

48/24. ALL ROOF SHEATHING SHALL BE FASTENED TO SUPPORTING MEMBERS W/10d COMMON NAILS @ 6" O.C. AT PANEL EDGES, AND 12" O.C. AT INTERMEDIATE SUPPORTS.

16 GA. AND HEAVIER STUDS SHALL HAVE A MINIMUM YIELD STRESS OF 50,000 PSI. 18

GA. AND LIGHTER STUDS AND TRACKS SHALL HAVE A MINIMUM YIELD STRESS OF 33,000

. STUDS AND TRACKS SHALL BE 18 GA. MINIMUM U.N.O. THEY SHALL BE MANUFACTURED

PROVIDE DOUBLE STUDS FOR FULL HEIGHT OF WALL EACH SIDE OF ALL OPENINGS UNLESS

OTHERWISE NOTED. WELD STUDS TO EACH OTHER WITH 1 1/2" LONG 1/8" FILLET WELDS

. REFER TO PLANS AND DETAILS FOR CONNECTION OF STUD WALLS TO FOUNDATION, FLOOR

SHOP DRAWINGS SHALL BE REVIEWED BY CONTRACTOR TO VERIFY THAT SUBMITTAL IS

DRAWINGS CREATED BY THE ENGINEER OF RECORD CANNOT BE REPRODUCED AND/OR

USED AS A SHOP DRAWING SUBMITTAL. SHOP DRAWING SUBMITTALS SHALL INCLUDE THE

SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 1705 OF IBC

AND THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE

WITH THE LISTED 2012 INTERNATIONAL BUILDING CODE SECTIONS/LOCATIONS:

DISCREPANCIES ARE TO BE REPORTED TO THE ARCHITECT IMMEDIATELY.

INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED UNDER SECTION

ALL DIMENSIONS ON STRUCTURAL DRAWINGS TO BE CHECKED AGAINST ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS BY THE GENERAL CONTRACTOR AND ANY

THE CONTRACTOR SHALL ASSUME RESPONSIBILITY, UNRELIEVED BY REVIEW OF SHOP

DRAWINGS OR PERIODIC OBSERVATION OF CONSTRUCTION. FOR COMPLIANCE WITH THE

CONTRACT DOCUMENTS, FOR FABRICATION PROCESSES AND CONSTRUCTION TECHNIQUES,

1705. THE FOLLOWING AREAS OF WORK REQUIRE SPECIAL INSPECTIONS IN ACCORDANCE

AT 12" O.C. EACH SIDE. PROVIDE STUD TRACK AT EACH HEAD AND SILL.

SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED ELECTRONICALLY.

COMPLETE PRIOR TO SUBMITTING TO ARCHITECT/ENGINEER.

32/16. ALL WALL SHEATHING SHALL BE FASTENED TO SUPPORTING MEMBERS W/ 8d

SPECIFICATIONS, LATEST EDITION. SHEATHING SHALL BE CONTINUOUS OVER THREE

ROOF & WALL SHEATHING

LIGHT GAGE METAL FRAMING

OR ROOF.

FOLLOWING:

SPECIAL INSPECTIONS

<u>MISCELLANEOUS</u>

A. CONCRETE MIX DESIGN

C. STRUCTURAL STEEL

E. ROOF SHEATHING

B. FOUNDATION REINFORCING BARS

F. TRELLIS SYSTEM & CALCULATIONS

G. LAMINATED VENEER LUMBER (LVL) H. SAWN LUMBER AND CONNECTORS

D. WOOD - SECTION 1705.5

3. DO NOT SCALE THE DRAWINGS.

D. OPEN WEB JOISTS AND CALCULATIONS

A. SOILS - SECTION 1705.6 PER TABLE 1705.6

C. STEEL - SECTION 1705.2 (SEE AISC 360.10)

AND FOR SAFE CONDITIONS ON THE JOB SITE.

B. CONCRETE - SECTION 1705.3 PER TABLE 1705.3

BY DIETRICH INDUSTRIES, INC. OR APPROVED EQUAL.

ADJACENT SPANS MINIMUM.

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STRUCTURAL NOTES

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FOUNDATION NOTES THE FOUNDATION DESIGN OF THIS BUILDING WAS BASED ON THE FOLLOWING CRITERIA:

RISK CATEGORY: II

SITE CLASS: C

G. FLOOD LOAD: N/A

H. SPECIAL LOADS: N/A

IMPORTANCE FACTOR: 1.00

SDS = 0.295, SD1 = 0.175

PLYWOOD SHEAR WALLS (R = 6.5)

DESIGN BASE SHEAR = SEE CALCULATIONS

SS = 0.369, S1 = 0.160

DESIGN CATEGORY: C

A. GROUND IMPROVEMENT REQUIRED PER VIBRATORY STONE COLUMN ENGINEER. FOUNDATION DESIGN SUBJECT TO CHANGE PENDING REVIEW OF GROUND IMPROVEMENT CONSTRUCTION DOCUMENTS. ENGINEER OF RECORD SHALL REVIEW AND APPROVE

STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE (R = 3.0)

Cs1: 0.045 (LONGITUDINAL DIRECTION) Cs2: 0.098 (TRANSVERSE DIRECTION)

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

- GROUND IMPROVEMENT DRAWINGS PRIOR TO CONSTRUCTION. B. ALLOWABLE SOIL BEARING CAPACITY = PENDING GROUND IMPROVEMENT ENGINEER C. RECOMMENDED BY PROFESSIONAL SERVICE INDUSTRIES, INC. IN THEIR REPORT #0040448-1 DATED OCTOBER 12, 2022.
- D. ANY FILL REQUIRED BELOW SLABS ON GRADE OR FOOTINGS SHALL BE COMPACTED AS REQUIRED BY THE SOILS REPORT NOTED IN ITEM #2.
- 2. ALL EXTERIOR FOOTINGS SHALL EXTEND BELOW THE MAXIMUM ANTICIPATED DEPTH OF FROST.
- . THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OR ENGINEER OF RECORD IMMEDIATELY IN THE EVENT THAT THE SOILS CONDITIONS ENCOUNTERED VARY FROM THOSE SHOWN ON THE BORING LOGS.
- . ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY A SOILS TESTING LABORATORY PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE AND REINFORCING

- . ALL CONCRETE SHALL BE IN ACCORDANCE WITH THE "AMERICAN CONCRETE INSTITUTE BUILDING CODE" (ACI 318) AND WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301) LATEST EDITIONS.
- 2. ALL NORMAL WEIGHT CONCRETE (145 PCF) SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI (3500 PSI FOR SLABS).
- . ALL CONCRETE SUBJECT TO EXTERIOR EXPOSURE SHALL BE AIR ENTRAINED AS RECOMMENDED BY ACI 318.
- 4. TEST CYLINDERS SHALL BE MADE AND TESTED AS OUTLINED IN CHAPTER 16 OF ACI-301.
- REINFORCING BARS SHALL BE DEFORMED BARS OF NEW BILLET STEEL CONFORMING TO ASTM A-615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. ALL REINFORCING AND ACCESSORIES SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI STANDARD 315 AND 315R.
- PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT POSITIONS SHOWN ON THE PLANS AND DETAILS. PLASTIC COATED ACCESSORIES SHALL BE USED IN ALL EXPOSED CONCRETE WORK.
- . THE GENERAL CONTRACTOR SHALL CHECK WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND THE SUB-CONTRACTORS FOR OPENINGS, SLEEVES, ANCHORS, HANGERS, INSERTS, SLAB DEPRESSIONS AND OTHER ITEMS RELATED TO THE CONCRETE WORK AND SHALL ASSUME RESPONSIBILITY FOR THEIR PROPER LOCATION.

- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN CONFORMANCE WITH THE AISC360 "SPECIFICATION FOR STRUCTURAL STEEL". SEISMIC DESIGN OF STRUCTURAL STEEL STRUCTURES SHALL CONFORM TO AISC 341.
- 2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:

A. ANCHOR RODS F1554, GRADE 36 B. HIGH STRENGTH STRUCTURAL BOLTS A325-N U.N.O.

C. STRUCTURAL SHAPES (W) A992

D. STRUCTURAL SHAPES (M, S, C, MC, PLATES) A36

E. STRUCTURAL SHAPES (HP) A572

F. STRUCTURAL TUBING (HSS) A500 GRADE B

3. ALL WELDING ELECTRODES SHALL BE E70-XX. ALL SHOP AND FIELD WELDING SHALL BE MADE IN ACCORDANCE WITH A.W.S. D1.1 "CODE FOR WELDING IN BUILDING CONSTRUCTION" AND SHALL BE MADE BY CERTIFIED WELDERS.

G. STRUCTURAL ANGLES A36

- AMINATED VENEER LUMBER (LVL) ALL BEAMS SHALL BE MANUFACTURED WITH LAMINATED VENEER LUMBER AND WATERPROOF ADHESIVES.
- 2. SIZE, MANUFACTURER & SERIES OF ALL LVL MEMBERS SHALL BE AS SHOWN ON DRAWINGS.
- ANY SUBSTITUTIONS MUST BE APPROVED IN WRITING BY ENGINEER OR ARCHITECT OF
- PROVIDE 3" MINIMUM BEARING OR AS SPECIFIED ON PLANS. REFER TO PLANS FOR FASTENING OF MULTIPLE PIECE BEAMS.

- OPEN WEB WOOD JOISTS OPEN WEB WOOD JOISTS SHALL BE MANUFACTURED WITH MACHINE STRESS RATED TOP AND BOTTOM CHORDS. WEBS SHALL BE TUBULAR STEEL MEMBERS PER MANUFACTURERS' SPECIFICATIONS.
- . SIZE, MANUFACTURER & SERIES OF ALL OPEN WEB JOISTS SHALL BE AS SHOWN ON DRAWINGS. ANY SUBSTITUTIONS MUST BE APPROVED IN WRITING BY ENGINEER OR ARCHITECT OF RECORD.
- PROVIDE 3 1/2" MINIMUM BEARING OR AS SPECIFIED ON PLANS. SHIM AS REQUIRED TO PROVIDE FULL BEARING AND LEVEL SUPPORT
- 4. DO NOT CUT TOP OR BOTTOM CHORDS.
- 5. ALL HANGERS AND FRAMING CONNECTORS SHOWN ARE MANUFACTURED BY SIMPSON STRONG TIE. ANY SUBSTITUTIONS MUST BE APPROVED IN WRITING BY ENGINEER OR ARCHITECT OF RECORD.
- 3. REFER TO PLANS FOR WEB STIFFENER AND CONCENTRATED LOAD REQUIREMENTS.
- REFER TO MANUFACTURERS' INSTALLATION GUIDE FOR JOIST BRACING DURING ERECTION. REFER TO MANUFACTURERS' INSTALLATION GUIDE FOR JOIST BRIDGING REQUIREMENTS.